











CONCRETE PLACED DIRECTLY IN CONTACT WITH GROU ALL BAR SIZES FACE OF CONCRETE ELEMENT EXPOSED TO WEATHER #5 BARS OR SMALLER ALL OTHER BAR SIZES FACE OF CONCRETE ELEMENT NOT EXPOSED TO WEAT SLABS, CAST IN PLACE WALLS BEAMS AND COLUMNS 1-1/2" STAIRS - ONE AND TWO FAMILY RESIDENCES DESIGN LIVE LOAD = 40 PSF EPOXY SHALL BE HILTI HIT HY 150 MAX (NOA#09-0224.06). CONCRETE SCREWS SHALL BE HILTI KWIK-CON II (NOA#07-0924 EXPANSION BOLTS SHALL BE HILTI KWIK BOLT 3 (NOA#06-0810. CONTRACTING NOTES COMPONENTS & CLADDING DESIGN WIND PRESSURES: WALLS: 10'-0" HEIGHT GENERAL ALL INFORMATION THAT IS STRUCTURAL IN NATURE SHOWN IN THESE STRUCTURAL SHEETS SHALL TAKE PRECEDENCE OVER STRUCTURAL ITEMS IN OTHER TRADES' PLANS. EXCEPTION: NOTES IN OTHER TRADES' PLANS ADDRESSING SPECIAL ITEMS (SUCH AS MINIMUM CONCRETE STRENGTH FOR FIRE RATING) SHALL GOVERN WHEN MORE STRINGENT THAN HERE SHOWN. CONCRETE PLACEMENT ACI347.2R-10, 4.5: NO WORKER SI PLACEMEN CEMENT SHALL BE TYPE I CONFORMING TO ASTM C-150. NOMINAL MAXIMUM COARSE AGGREGATE SIZE TO BE 3". ALL CONCRETE TO BE READY MIX CONCRETE TO ACHIEVE A 28 DAY COMPRESSIVE STRENGTH OF 5000 PSI WATER TO CEMENT RATIO OF 0.4) FOR ALL STRUCTURAL ELEMENTS (INCLUDING, BUT NOT LIMITED TO FOOTINGS, SLABS, BEAMS, COLUMNS). CUTTING HOLES IN PRE-ENGINEERED FLOOR SYSTEMS SHALL BE COORDINATED BY THE GENERAL CONTRACTOR AND THE SYSTEM SUPPLIER BEFORE PRODUCTION. DEFLECTION DESIGN CRITERIA ALL OTHER TRADES PLANS SHALL BE VERIFIED TO IDENTIFY THE LOCATION OF PIPES, PENETRATIONS, SLEEVES, ETC. CONCRETE REINFORCEMENT BARS, STIRRUPS AND TIES SHALL BE IN ACCORDANCE WITH ASTM A-615 (GRADE 60). REINFORCING BARS THAT ACT AS LAP DOWELS AND ARE TO BE WELDED TO STRUCTURAL STEEL SHALL CONFORM TO ASTM A-706 (GRADE 60). REINFORCING BARS CONTAINING A LIGHT COAT OF RUST SHALL BE ALLOWED. ACI HOOKS TO BE PROVIDED FOR ALL REQUIRED CONDITIONS PER DETAILS PRESENTED IN THIS PROJECT. CONCRETE REINFORCEMENT (ACI 318-08, 315-08, LRFD) ANCHORING SPECIFICATIONS ALL DETAILS AND SECTIONS SHOWN ARE INTENDED TO BE TYPICAL UNLESS OTHERWISE SPECIFIED. DETAILS THAT REFERENCE SPECIFIC SCHEDULES ARE INTENDED TO BE SCHEDULES SHALL TAKE PRECEDENCE IN ALL CONFLICTING ITEMS. ALL ELEMENTS INTENDED TO ORIGINATE FROM OTHER TRADE'S DRAWINGS (ELEVATIONS, PARTITION LOCATIONS, ETC) HAVE BEEN INCLUDED FOR THEIR RELATION TO STRUCTURAL ITEMS ONLY. REFER TO THE CORRESPONDING TRADE'S PLANS FOR FINAL LAYOUTS AND DETAILS. DO NOT USA A SCALE TO DETERMINE THE DIMENSIONS OF ANY PLAN OR DETAIL DRAWINGS. IF THE DIMENSION NEEDED IS UNAVAILABLE, PLEASE CONTACT THIS OFFICE. PROJECT PLANS, DETAILS & ARCHITECTURAL BACKGROUNDS WIND LOADING (ASCE 7-10) WIND SPEED=170 MPH (3 SECOND GL GRAVITY LOADING (ASCE 7-10, CHAPTER, TABLE C3-1 & C3-2) SEE GRAVITY LOADING CHART FOR DESIGN LOADS. ALL STRUCTURAL ELEMENTS FOR THIS PROJECT HAVE BEEN DESIGNED PER FLORIDA THIS IMPLIES THE USE OF THE BELOW LISTED CODES (FBC TAKES PRECEDENCE OVER PROJECT SPECIFIC CODES USED: ALL FORMS SHALL BE CLEANED WITH COMPRESSED ETC, PRIOR TO THE PLACEMENT OF ANY CONCRETE ALL FRESH CONCRETE TO EXISTING CONCRETE CONTACT FACES EPOXY BONDING AGENT APPLIED AS PER THE MANUFACTURER'S: ALL EXTERIOR SLAB REBAR SHALL BE SEPARATED BY PLASTIC CHAIRS FROM ANY CONCI WELDED REINFORCEMENT SHALL CONFORM TO AWS D1.4 OR BE ASTM A706 GRADE 60. ANCHOR EMBEDMENT LENGTH INCLUDE STRUCTURAL CONCRETE ONLY (DOES NOT INCL ALL CONCRETE SURFACES TO BE KEPT IN A MOIST CONDITION FOR 7 DAYS AFTER POUR 5000 PSI CONCRETE_(ACI 318-08, 315-08, LRFD) STRUCTURAL NOTES)LUMN AND FILLED CELL LAP SPLICES SHALL BE 48 BAR DIAMETERS. IE CONTRACTOR SHALL ALLOW FOR AN ADDITIONAL 1% AMOUNT OF REBAR TO BE USED AT THE DISCRETION THE STRUCTURAL INSPECTOR IN ORDER TO RECTIFY OR STRENGTHEN ANY CONDITIONS FOUND TO BE: -0" HEIGHT ZONE 4 = +46.20 PSF, -50.56 PSF ZONE 5 = +46.20 PSF, -58.39 PSF GENERAL CONTRACTOR MUST FAMILIARIZE HIMSELF WITH THE STRUCTURAL PLANS AND HOW THEY INTERACT WITH OTHER SPECIALTIES PLANS (ARCHITECTURAL, M.E.P., ETC). ALTHOUGH TRADE COORDINATION HAS BEEN PERFORMED, THIS OFFICE SHALL BE CONTACTED IMMEDIATELY IN CASE THE CONTRACTOR FINDS ANY MISSING, AMBIGUOUS, UNCLEAR OR CONFLICTING INFORMATION. THE CONTRACTOR SHALL ALWAYS KEEP ON SITE THE LATEST REVISION OF THE SIGNED AND SEALED STRUCTURAL PLANS, APPROVED SHOP DRAWINGS AND PRODUCT APPROVALS. ALL OPENINGS BELOW 60'-0" TO BE PROTECTED FROM IMPACT OF WINDBOURNE DEBRIS GENERAL CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL ACTIVITIES RELATED TO WORK SAFETY. CONSTRUCTION LOADS SHOULD BE CAREFULLY DISTRIBUTED TO AVOID LOADING ANY MEMBERS IN EXCESS OF THEIR CAPACITY. THE APPROVAL BY THE STRUCTURAL INSPECTOR OF ANY WORK DOES NOT RELIEVE THE CONTRACTOR FROM COMPLYING WITH THE FLORIDA BUILDING CODE AND THE APPROVED STRUCTURAL PLANS. HEIGHT ZONE 4 = +45.17 PSF, -49.53 PSF ZONE 5 = +45.17 PSF, -56.33 PSF SIGNED BY THE DELEGATED ENGINEER (IF / TOTAL LOAD: SPAN / 360 REINFORCED CONCRETE ELEMENTS: LIVE I WALL / MULLIONS: L / 175 OTHER ELEMENTS: LIVE LOAD: SPAN / 480 HEIGHT ZONE 4 = +46.81 PSF, -51.17 PSF ZONE 5 = +46.81 PSF, -59.61 PSF PRESSURE COEFFICIENT= +/- 0.18 YPE= ENCLOSED, RIGID REINFORCEMENT IN STRUCTURAL SLABS SHALL BE INSTALLED IN ALL AREAS OF THE SLAB AND NICULAR TO THE MAIN REINFORCEMENT. NITTED TO BE ADDED TO THE CONC 2" SED TO WEATHER OR C ³/₄" 1-1/2" SHOP DRAWINGS: FLOOR AND ROOF SYSTEM SHOP DRAWINGS WILL NOT BE STAMPED UNSTAMPED AND: GENERAL CONTRACTOR HAS VERIFIED: -FIELD DIMENSIONS -AS BUILT VARIATIONS WHICH MAY AFFECT CONNECTION DETAIL-GENERAL COMPLIANCE WITH THE APPROVED PLANS -ARCHITECT HAS VERIFIED: -INTERFERENCE BETWEEN THE SHOP DRAWING ELEMENTS AND AND ANY OTHER GEOMETRICAL CEILING FEATURES. -LENGTH OF OVERHANG, RIDGE LINES AND ROOF PITCH-MINIMUM AREAS DESIGNATED FOR STORAGE SPACES. -EXTERIOR WALL RECESSES ALL STRUCTURAL STEEL SHALL COMPLY WITH AISC LRFD 13TH EDITION. STEEL BEAMS SHALL COMPLY WITH ASTM A992 (GRADE 50). STEEL RECTANGULAR / SQUARE COLUMN SECTIONS - ASTM A-500 (Fy=46 KSI), GRADE B. STEEL CIRCULAR COLUMN SECTIONS - ASTM A-500 (Fy=42 KSI), GRADE B. STEEL ANGLES, C-CHANNELS, MC-CHANNELS, AND S-SHAPES SHALL CONFORM TO ASTM A36. ALL STEEL BOLTS AND THREADED RODS, UNLESS NOTED OTHERWISE ON THE PLANS TO BE A A AND INSTALLED TO A SNUG TIGHT CONDITION (DEFINED AS BEING TIGHTENED BY THE FULL WORKER USING A REGULAR SPUD WRENCH). BOLT NUTS TO BE ASTM A563. ALL STRUCTURAL STEEL (NOT INCLUDING REBAR) TO HAVE A SHOP COAT OF RUST INHIBITIVE INSTALLATION A FIELD TOUCH UP SHALL BE APPLIED. ANY STRUCTURAL STEEL THAT IS TO BE EXTERIOR TO THE BUILDING ENVELOPE SHALL BE HC ALL SHEAR STUDS SHALL BE TYPE B, ASTM A108 (Fu = 65 ksi) ALL CABLES SHALL BE STAINLESS STEEL 6x19 WIRE ROPE - TYPE 304, IN COMPLIANCE WITH A: PROVISIONS. GENERAL CONTRACTOR TO PROVIDE THIS OFFICE WITH SHOP DRAWINGS AND CALCULATIONS SIGNED AND SEALED BY THE DELEGATED ENGINEER FOR APPROVAL. CONTRACTOR TO ANTICIPATE FOR TIME OF REVIEW, ADJUSTMENT AND RESUBMITTAL. SAID DRAWINGS SHALL BE ENGINEERED TO COMPLY WITH THE DESIGN PARAMETERS SPECIFIED IN THESE PLANS BUT IN NO CASE LESS THAN THE MINIMUMS REQUIRED BY THE ABOVE MENTIONED CODES. DESIGN PARAMETERS USED AND ALL LOADS FROM SPECIALTY MEMBERS ACTING AGAINST THE SUBSTRUCTURE SHALL BE CLEARLY IDENTIFIED IN THE REVIEW PACKAGE. DESIGN SPECIFICATIONS FOR METAL PLATE CONNECTED WOOD TRUSSES SPECIFICATIONS FOR METAL PLATE CONNECTED PARALLEL CHORD ROOF WOOD TRUSSES: COMMENTARY AND RECOMMENDATIONS (BWT 76); HANDI COMMENTARY AND RECOMMENDATIONS (HIB-91). ACCOUNTING SEPARATELY FOR EACH CLASS OF CONCRETE AND EACH DAY, ONE TEST (CONSISTING OF BREAKING (2) 6"Ø DIAMETER x 12" HIGH CONCRETE CYLINDER SAMPLES, MOLDED AS PER ASTM C31 AND TESTED PER ASTM C39) SHALL BE PERFORMED ACCORDING TO THE MOST FREQUENT OF: -ONCE A DAY -ONCE EVERY 150 CUBIC YARDS -ONCE EVERY 5000 SQUARE FEET OF SLAB OR WALL SURFACE AREA MASONRY UNITS SHALL BE ASTM C-90 TYPE II WITH MINIMUM COMPRESSIVE STRENGTH OAREA OF INDIVIDUAL UNITS PER FBCR 606.4. ALL CMU SHALL BE LAID IN A FULL BED OF MOBOND U.O.N. ALL REINFORCING STEEL SHALL BE MANUFACTURED FROM HIGH STRENGTH CONFORMING TO ASTM DESIGNATION A-615 GRADE 60. DO NOT CUT THE HEADS OR NUTS OF ANY BOLTS THAT WITHOUT PRIOR CONSULTATION WITH THIS OFFICE. \cdot STRUCTURAL PRE-ENGINEERED WOOD ROOF TRUSSES, GIRDERS THEM ARE TO BE PROVIDED BY THE DELEGATED ENGINEER (REGI AND SEALED SHOP DRAWINGS AND CALCULATIONS FOR REVIEW FORMPLIANCE WITH INITIAL LAYOUT AND DESIGN PARAMETERS. ALL PILE CONCRETE COVER TO REINFORCEMENT TO BE 3". ALLOWABLE PILE LOCATION TOLERANCE TO BE 2". GENERAL CONTRACTOR TO RETAIN A FLORIDA LICENSED EINSTALLATION. PILE INSTALLATION METHODOLOGY PROVIDED BY THE ABOVE MENTIONED SOILS REPOR PRECEDENCE TO NOTES HEREIN. PILES TO BE FORMED BY DRILLING WITH A HOLLOW ST CONCRETE TO BE PUMPED AS THE AUGER IS REMOVED WHILE STILL ROTATING. VERIFY , CONCRETE VOLUME WITH EXPECTED VOLUME TO ENSURE CONCRETE IS NOT EVACUATII CAST IN PLACE CONCRETE PILES (AUGER PILES). ALL AUGURED CAST IN PLACE CONCRETE FILLED PILES SHALL BE REINFORCED WITH (4) HEIGHT, CONFINED BY #3 TIES @ 12" O/C FOR THE UPPER 14', BALANCE @ 48" O/C. PILES AXIAL COMPRESSION ALLOWABLE / WORKING CAPACITY. DEEP FOUNDATION (GRADE BEAM DESIGN: LRFD, PILE CAPACITY: 7 DAY, 14 DAY AND 28 DAY TEST REPORTS SHALL BE SUBMITTED TO THE STRUCTURAL E FOR REVIEW. THE CLIENT SHALL RETAIN A REPUTABLE TESTING COMPANY TO BE ON SIT CONCRETE POUR AND INDEPENDENTLY COLLECT CYLINDERS AND PERFORM COMPRES **CONCRETE TESTING**QUALIFIED FIELD TESTING TECHNICIANS SHALL PREPARE SPECIMENTS REQUIRED FOR CONDITIONS AND RECORD THE TEMPERATURE OF THE FRESH CONCRETE WHEN PREP, LABORATORY STRENGTH TESTS. TO BE DESIGNED ACCORDING TO ASCE 7-05 SECTION 4.5.1: ABLE TO RESIST MOST STRIN OF 50 LBF/FT OR 200 LBF, APPLIED IN ANY DIRECTION AT THE TOP. INTERMEDIATE PICKE RESIST A SINGLE POINT LOAD OF 50 LBF. ALL CONCRETE MASONRY, TIE BEAMS, TIE COLUMNS, ETC SHALL BE INSPECTED BY A RE BEFORE CONCRETE POUR. UPON INSPECTION APPROVAL, AN ENGINEERING SIGNED, SE. LETTER WILL BE PROVIDED TO THE GENERAL CONTRACTOR AS REQUIRED BY THE AHJ. THIS PROJECT IS DESIGNED AS ENGINEERED UNIT MASONRY. STRUCTURAL DESIGN SHA WITH ACI 530-05/ASCE 5-05/TMS 402-05, BUILDING CODE REQUIREMENTS FOR MASONRY : COMMENTARY ON BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES. MASONRY (ACI 530-08, ASD) ALL STEEL BEAMS CONNECTIONS TO EMBED PLATES ON CONCRETE SHALL BE DONE AF-REACHED 70% OF THE SPECIFIED COMPRESSIVE STRENGTH (VERIFY WITH THE 7 DAY CC BREAK RESULTS). STRUCTURAL STEEL (AISC LRFD) TRUSS AND GIRDER CONNECTIONS TO THE MAIN STRUCTUF BY THE ENGINEER OF RECORD. WOOD TRUSSES (PRE-ENGINEERED) BOTTOM CHORD ABLE TO SUPPORT A SINGLE CONCENTRATED POINT PER FBC 1615.2.1. REPORT ON SOILS AND FOUNDATION INVESTIGATION PREPARED BY NUTTING ENGINEERS FLORIDA,INC. - N°. 16650.1 - (03/18/13) IF LESS THAN 50 CUBIC YARDS ARE USED FOR A SINGLE CLASS SUFFICIENT EVIDENCE IS PROVIDED TO THE BUILDING OFFICIAL HANDRAILS AND GUARDRAILS GROUT SHALL BE A HIGH SLUMP MIX (8"-11") IN ACCORDANCE WITH ASTM SPECIFICATION MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI. NO TESTING FOR GROUT STRENGTH IS ∍ROJECT. ALL STEEL BEAM CONNECTIONS ARE INTENDED TO SIT DIRECTLY ON TOP OF TOP PLATE TUBE COLUMNS, NOT TO THE SIDE OF STEEL TUBE COLUMNS). DO NOT CUT TOP PLATES BE FILLED WITH GROUT. SIGN, FABRICATION, AND INSTALLATION OF WOOD TRUSSES AND SHEET METAL CONNCORDANCE WITH THE FOLLOWING STANDARDS. MORTAR SHALL BE TYPE S IN ACCORDANCE WITH ASTM SPECIFICATION C-270 WITH A MINIMUM MPRESSIVE STRENGTH OF 1800 PSI AT 28 DAYS. NO TESTING FOR MORTAR STRENGTH IS REQUIRED FOR THIS DIECT. MORTAR MAY BE RE-TEMPERED WITH WATER TO REGAIN WORKABILITY FOR NO MORE THAN 2.5 JRS AFTER INITIAL MIXING. SHOP AND FIELD WELDING SHALL BE PERFORMED BY WELDERS QUALIFIED, AS DESC DING SOCIETY'S STANDARD QUALIFICATION PROCEDURE" (ASW D1.1), TO PERFORM: UIRED. ALL STEEL WELDING RODS SHALL BE E70XX ELECTRODES. SITE PREPARATION AND EXCAVATION WORK IS TO BE PERFORMED SSES TO BE DESIGNED BY THE DELEGATED ENGINEER USING SERVICE LIVE LOAD OF 20 RS AND ANY CONNECTIONS GISTERED IN FLORIDA) IN T W BY THE ENGINEER OF RE RCED UNIT MASONRY SCRIBED BY MSJC TABLE RTIFICATES OF 3 WITH ASTM C90, THE DEFORMED PACES, LADDER TYPE S AND CONCRETE ONLY) AT EVERY 4" INTO ALL CONCR OF 1900 PSI ON NET MORTAR IN RUNNING H BILLET STEEL REQUIRED FOR THIS GENT OF LINEAR LOAD 'S TO BE ABLE TO ALED AND DATED LL BE IN ACCORDANCE TRUCTURES AND THE TES OF STEEL BUCKETS RIBED IN "AMERICAN THE TYPE OF WORK ITIVE PAINT. AFTER #6's PERIMETER, FULL TO ACHIEVE A 35 TON ASD) AGINEER OF RECORD E AT THE TIME OF SIVE TESTING. ER CONCRETE HAS NCRETE SPECIMEN OF STEEL COLUMNS TO ECTORS SHALL BE IN ABBREVIATION LEGEND: ADDL - ADDITIONAL AB - ANCHOR BOLT ACI - AMERICAN CONCRETE INSTITUTE AISI - AMERICAN IRON AND STEEL INSTITUTE AISC - AMERICAN INSTITUTE OF STEEL CONSTRUCTION ALUMINUM AA ADM 2005 (ASD) ALL ALUMINUM STRUCTURAL MEMBERS SHALL BE ALLOY T6061-T6. ALL BOLTS USED IN CONNECTIONS OF ALUMINUM MEMBERS SHALL BE (PER SPECIFICATION SECTION 5.2.1) -ALUMINUM BOLTS CONFORMING TO T6061-T6 WITH A MINIMUM OF 2 MIL THICK ANODIC COATING. -STRUCTURES OF INTERIOR WEATHER EXPOSURE: CARBON STEEL: A307 HOT DIPPED GALVANIZED TO ASTM A153 OR ELECTRO GALVANIZED TO ASTM B633. -STRUCTURES OF EXTERIOR WEATHER EXPOSURE: OUTDOOR: 300 SERIES STAINLESS STEEL. ALL SCREWS SHALL BE 300 SERIES STAINLESS STEEL. SYMBOL LEGEND ABOVE 6T-OUTER 4@6B-OUTER ALUMINUM ALLOWABLE STRESS D ANCE, REMAINING BASE PLATE TTOM CANTILEVERED CAST IN PLACE ENTER LINE JENERAL CONTRACTOR JEADED STUD / NELSON STUD JOLLOWCORE JOT DIPPED GALVANIZED LOW STRUCTURAL SECTION 000 PER LINEAR FOOT NOCK-OUT 0 BOVE ' FF +0'-0" +55.06 -88.88 BELOW S7 翿 **ECCENTRIC BASE PLATE** SCALE: 1-1/2" = 1' NEW CONCRETE SLAB CMU WALL ONE LEVEL BELOW PARTITION STEEL COLUMN REVISION NUMBER - SEE TITLE BLOCK FOR DATE ROOF RIDGE LINES WELD SYMBOLS NEW CONCRETE FOOTING NEW CONCRETE BEAM FILLED CELL - SEE CONC. COL. CONCRETE COLUMN - SEE CONC. COL SCHED. FLOOR STEP THICK DASHED LINE REPRESENTS OPENING ON STORY BELOW REINFORCED MASONRY WALL CONCRETE WALL VERTICAL REINFORCEMENT. SEE FOUNDATION / FRAMING PLANS FOR LOCATIONS ELEVATION MARKER - SEE ARCH PLANS FOR +0'-0" UPLIFT FORCES & CONNECTORS 14"Ø AUGER CAST PILE TO BE CENTERED UNDER GRADE BEAM. SEE STRUCTURAL NOTES FOR REINF REVISION CLOUD STEEL COLUMN BELOW FLOOR LEVEL SECTION MARK - DETAIL NUMBER / SHEET NUMBER TIE BEAM TRANSITION SYMBOL CONCRETE BEAM CALLOUT - SEE SCHEDULE STEEL REIN 4@6: -B: -T: -OUTER: -INNER: -TEMP: JOISTS OR TRUSSE #4's @ 6" O/C BOTTOM TOP OUTERMOST LAYER INNERMOST LAYER TEMPERATURE AND SHRINKA - PLATE - RING SHANK NAIL - REINFORCED CONCRETE - SELF DRILLING SCREW STANDARD - STEEL ANGLES WITH SHOI - LOAD AND RESISTANCE - LIVE LOAD - STEEL ANGLES WITH LC STEEL JOIST INSTITUTE STEEL DECK INSTITUTE SHEAR WALL SHORT WAY 0 - SEE STRAP SCHED - CONNECTORS PER STEEL COLUMN SCHEDULE. STEEL COLUMN SEE FRAMING PLAN FOR SCHEDULE CODE CONCRETE BEAM SEE PLANS & SCHED. FOR DIM. & REINFORCING STEEL T/ SLAB FNDN PLAN S7 TEMPERATURE REINI PER FNDN PLAN. MAIN BOTTOM REIN PER FNDN PLAN. MAIN TOP REINFOF PER FNDN PLAN. GRADE BEAM - SEEPLANS & SCH FOR DIM. & REINFORCING STEEL VERTICAL REINF ☐ PER COLUMN SCHED LAP SPLICE= 48 BAR DIAMETERS STRUCTURAL **GRADE BEAM STEP** SCALE: 3/4" = 1' SCALE: 3/4" = 6" LAPPED AND TAPED -6 MIL. V.B. OVER TERMITE TREATED SOIL CONCRETE SCALE: 3/4" = 1' TIES PER CONC. COL SCHED. CMU WAL HORIZO SHALL E COLUMI AT ALL I SLAB COLUMN EDGE - 14" Ø AUGER CAST PILE EXTEND STEEL INTO GRADE BEAM 18" MIN. 3" PILE EMBEDMENT INTO GRADE BEAM - 6" LAPPED AND TAPED 6 MIL. V.B. OVER TERMITE TREATED SOIL (2) Z - BARS TOTAL MATCH GRADE BEAM REINFORCEMENT BAR DIAMETER DETAIL OOL WALL AND DESIGN BY OTHERS DETAIL 8"x8"x16" CONCRI MASONRY UNITS S7 REINFORCED FILLED CEL ထ္ SCALE: 1/2" = 1' THICK CMU LOW WALL REINFORCEMENT DETAIL CMU TYPICAL SCALE: 1/2" = 1' 3" CLEAR CUV. SCHED 48 BAR DIA LAP. SCHED. - 8" CMU KNOCKOUT BLOCK W/ (1) # 5 CONTINOUS TOP. 6 S7 1 1/2" CLR. HOOK UNDERNEATH TIES CONC. COL. ANCHORAGE. SCHED SCALE: 1/2" = -#3 TIES SEE COL SCHED. FARID ABUGATTAS, P.E. STRUCTURAL ENGINEER 7318 TEXAS TRAIL BOCA RATON, FLORIDA 33487 FLOOR SYSTEM PHONE: (954) 667 - 7803 FAX: (561) 665 - 5438

P.E. LICENSE # 72471, CA#29407 WWW.PROJECTCLASSIC.COM FARID@PROJECTCLASSIC.COM





