HURRICANE ENGINEERING & TESTING INC.





ISO 17025 Accredited Computer Controlled Product Testing Wind Load Design, Analysis & Evaluation



Static Wind Load Test ASTM E 72-14a

Strength Test of Panels for Building Construction Transverse Load-Horizontal Specimen

May 5, 2015

REPORT NUMBER:

HETI-15-5032

MANUFACTURER:

PROJECT CLASSIC STRUCTURAL ENGINEERING

7318 Texas Trail, Boca Raton, Florida 33487.

TEST LOCATION:

Hurricane Engineering & Testing Inc.

6120 NW 97th Avenue, Doral, Florida, 33178

NOTIFICATION NUMBER:

HETI14025 (MIAMI-DADE COUNTY, FLORIDA 10-1117.07 (MIAMI-DADE COUNTY, FLORIDA)

LAB. CERTIFICATION No.: IAS. CERTIFICATION No.:

TL-296 (ISO 17025-05)

FBC ORGANIZATION No:

TST1691

FBPE Certificate of Authorization Number: 6905

PRODUCT:

Composite Panels

(See Hurricane Engineering & Testing, Inc. marked Drawing).

PRODUCT SIZE:

Test # 7: 51" wide x 256" long x 8 3/16" deep (20'-8" Span)

PRODUCT DESCRIPTION:

3500 psi Concrete Covered Steel Panel (Reference Material Tensile Test

Report No. HETI-15-T303; Concrete Compression Test Report No. HETI-

15-C101)

DRAWING NO.:

Test #7: S4 by Project Classic Structural Engineering, dated 2/26/15

TEST WITNESSED BY:

Syed Waqar Ali, Ph.D. (HETI) Nasreen K. Ali, E.I. (HETI) Eugenio Rivera (HETI)

Mr. Rafael E. Droz-Seda, P.E. (HETI)



ULTIMATE PRESSURE SUMMARY

Report No.	Test No.	Sample Span	Ultimate Pressure
HETI-15-5032	7	20'-8"	240 psf
HETI-15-5033	7	20'-8"	160 psf
HETI-15-5034	7	20'-8"	180 psf

20'-8" Product Description

Each sample was constructed by attaching (2) separate panel halves on top of each other to create the hexagon shaped steel frame. The panels were setup to create a 1 5/8" camber at the center. The (2) panels were attached with (4) rows of (31) #10 x ¾" Hex Head Self Drilling Screws (HH SDS) located at 1 ½", 6 5/8", 12 ¾", 19 ½", 24 1/8", 30 5/8", 37", 44 ¾", 52 ¼", 64 5/8", 75 5/8", 85", 96 1/8", 104 ½", 111 ½", 124", 132 ½", 142 ¼", 151 5/8", 162 ½", 172 ¼", 182 ¾", 193 ¼", 201 3/8", 209 ½", 216 3/8", 222 ½", 228 ¾", 235 1/8", 240 ½", and 245 5/8" from the left end. Once the samples were constructed, a layer of 0.142" thick (6" x 6" square) steel lathing the size of the sample was laid on top Once the sample was completely constructed, an average layer of 2.06" of 3500 psi concrete was poured on top of the finished single panel sample.

Individual Panel Size: $25 \frac{1}{2}$ " w x 248 $\frac{1}{2}$ " long x 3 1/16" deep Double Finished Panel Size: $50 \frac{3}{8}$ " w x 248 $\frac{1}{2}$ " long x 6 1/8" deep

Composite Finished Panel Size: 51" w x 256" long x 8 3/16" deep

Corrugated Panel Thickness: 16ga (0.060" with coating)

Concrete: 3500 psi

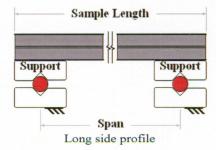
Test Procedure: Tested as per ASTM E 72 - 14a, Section 11.3.1.1

Note*: All composite panels manufactured with concrete resulted in a partial filling of the hexagonal cavity at each end of the panel. The minimum full hexagon fill is 8" and the maximum full fill of the cavity was 10". After the full fill of the cavity, the concrete tapers off to zero in 10" to 24". See photo below.



Apparatus: Uniformly distributed loading by vacuum pump acting below the specimen, a 2 mil plastic sheathing above the specimen and an airtight chamber as per section 11.3.1.3 of ASTM E 72 - 14a. The pressure was read with a water column manometer and a pressure transducer.

Supports: The samples were supported by rollers as per diagram below:



Deflection Gage: The deflection was measured using linear variable differential transformers (LVDT HETI-0172).

TEST RESULTS

Test # 7
PRODUCT SIZE: 20'-8"

(Test Date: February 26, 2015)

	Sample # 1	(Test Reference	No. HETI-1.	5-5032)
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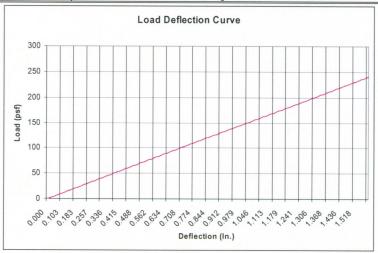
	Pressure (psf)	Deflection Location 1 (inches)	Set Loc.1	Recovery Loc. 1 (%)	Duration (seconds)
Starting Load	0.0	0.00	0.00	100	300
Load	60	0.45	0.00	100	300
Load	80	0.61	0.00	100	300
Load	100	0.71	0.00	100	300
Load	110	0.76	0.00	100	300
Load	120	0.84	0.00	100	300
Load	130	0.89	0.00	100	300
Load	140	0.96	0.00	100	300
Load	150	1.03	0.00	100	300
Load	160	1.07	0.00	100	300
Load	170	1.14	0.00	100	300
Load	180	1.21	0.00	100	300
Load	190	1.27	0.00	100	300
Load	200	1.34	0.00	100	300
Load	210	1.39	0.00	100	300
Load	220	1.50	0.01	99	300
Load	230	1.57	0.02	99	300
Load	240	1.76	0.03	98	300
Load	250	1.88	0.10	95	300
Failure Load	ilure Load Failed at 270 PSF, top surface buckled at center.				



(Test Date: February 26, 2015)

Sample # 2 (Test Reference No. HETI-15-5033)

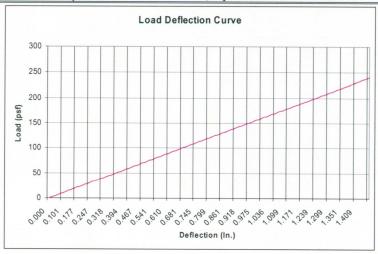
	Pressure (psf)	Deflection Location 1 (inches)	Set Loc.1	Recovery Loc. 1 (%)	Duration (seconds)
Starting Load	0.0	0.00	0.00	100	300
Load	100	0.83	0.04	95	300
Load	150	1.10	0.05	95	300
Load	200	1.52	0.12	92	300
Load	220	1.56	0.02	99	300
Load	240	1.84	0.16	91	300
Failure Load	Failed at 259 PSF, top surface buckled at center				



(Test Date: February 26, 2015)

Sample # 3 (Test Reference No. HETI-15-5034)

	Pressure (psf)	Deflection Location 1 (inches)	Set Loc.1	Recovery Loc. 1 (%)	Duration (seconds)
Starting Load	0.0	0.00	0.00	100	300
Load	100	0.78	0.05	94	300
Load	200	1.53	0.19	88	300
Load	220	1.49	0.05	97	300
Load	240	1.52	0.06	96	300
Failure Load	Failed at 260 PSF, top surface buckled at center				



Conclusion

The samples were structurally intact at the conclusion of this test.

NOTE: The above results were obtained using the designated test methods that indicates compliance with the performance requirements of the referenced specifications. This report does not constitute certification of the specimens tested.

STATEMENT OF INDEPENDENCE

The Hurricane Engineering & Testing, Inc., does not have, nor does it intend to acquire or will acquire, a financial interest in any company manufacturing or distributing products tested or labeled by the Hurricane Engineering & Testing, Inc. Hurricane Engineering & Testing, Inc., is not owned, operated or controlled by any company manufacturing or distributing products it test or labels.

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Resident Engeneer